

## PROJECT IDENTIFICATION

### CALCULATION IDENTIFICATION

## CAP DRAINAGE LAYER HYDRAULICS

## RECORD OF REVISION

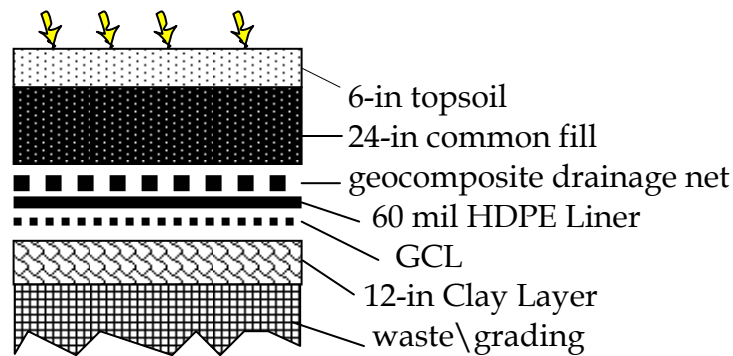
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## CAP DRAINAGE LAYER HYDRAULICS

### 1. GEOCOMPOSITE DRAINAGE NET HYDRAULICS

#### 1.1 Data input

-cap design:



- soil layer common fill permeability:  $k = 0.00001 \text{ cm/s}$  ( $1 \times 10^{-7} \text{ m/s}$ )
- Slope gradient and length for selected critical paths:
  - a) 5 % approx. 100 ft = 30 m
- optional 0.30 m thick sand drainage layer, typical permeability  
 $K_s = 0.03 \text{ cm/s} = 0.0003 \text{ m/s}$
- reduction factors for drainage composite
  - for intrusion  $RF_{in} = 1.5$
  - for creep  $RF_{cr} = 1.4$
  - for chemical clogging  $RF_{cc} = 1.2$
  - for biological clogging  $RF_{bc} = 1.6$
  - overall  $FS = 2$
  - Total  $FS = 8$
- criteria for Lateral Drainage for Final Cover Side Slope, Landfill Drainage System [www.landfilldesign.com](http://www.landfilldesign.com) Unit Gradient Method (see attached).

**CRA**

PROJECT NO: 042192-95

DESIGNED BY: A.W.

PROJECT NAME: Capping

CHECKED BY: R.H.

DATE : Oct10/06

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### 1.2 Required transmissivity of the geocomposite $Y_{ult}$

Required (ultimate) geocomposite transmissivities for selected paths, (length, permeability and slope) have been calculated utilizing software program , Unit Gradient Method, (see attached).

a)  $Y_{ult} = 0.00048 \text{ m}^2/\text{s}$  for 5% slope



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### 1.3 Available transmissivity of the geocomposite $Y_{avail}$

Available transmissivities for GSE Fabrinet 250 mil geocomposite product, according to attached manufacturer chart, Fig.A-6 for given (design) gradients and normal pressure of approximately 1000 psf at given cap design configuration.

$$a) Y_{avail} = 0.001 \text{ m}^2/\text{s for 5\% slope}$$

### 1.4 Infiltration into sand drainage layer

Calculated infiltration flow, based on unit length along the slope  $L_s$ , given soil cover permeability  $K_{cover}$  and vertical seepage gradient = 1.

$$Q_{infil} = L_s \times K_{cover} \times 1 \text{ (m}^3/\text{s per m width, per linear m)}$$

$$Q_{infil} = 30\text{m} \times 0.0000001 \text{ m/s} \times 1 = 0.000003 \text{ m}^3/\text{s}$$

### 1.5. Available sand drainage layer hydraulic capacity $Q_{avail}$

Calculated available sand drainage layer hydraulic capacity, based on sand drainage layer permeability  $K_{sand}$ , vertical gradient slope  $i$ , and drainage layer cross-sectional area  $A$ .

$$Q_{sand} = K_{sand} \times i \times A \text{ (m}^3/\text{s per m width)}$$

$$Q_{sand} = 0.0003\text{m/s} \times 0.05 \times (0.3 \text{ m} \times 1 \text{ m}) = 0.0000045 \text{ m}^3/\text{s}$$

### 1.6 Conclusion

#### a) Geocomposite drainage layer :

According to the results as shown in 1.2 and in 1.3, available transmissivities of GSE product are fully satisfactory, and no lateral drains are required. Flow generated from the cap infiltration will be fully contained within the drainage layer, providing that the cover soil has a permeability of  $1 \times 10^{-5}$  cm sec or less.

$$Y_{\text{avail}} = 0.001 \text{ m}^2/\text{s} > Y_{\text{ult}} = 0.00048 \text{ m}^2/\text{s} \text{ for } 5\% \text{ slope}$$

$$\text{Factor of safety, } F_s = Y_{\text{avail}}/Y_{\text{ult}} = 0.001/0.00048 = 2.08$$

( including previously applied factors for clogging as in part 1.1 on page 2)

#### b) Optional sand drainage layer:

According to the results as shown in part 1.4 and in 1.5, no lateral/relief drain will be required.

Flow generated from the cap infiltration will be fully contained within 0.3 m of the sand drainage layer, providing that the cover soil material and sand material will have permeability as assumed.

However factor of safety is only :

$$F_s = Q_{\text{sand}}/Q_{\text{infilt}} = 0.0000045 / 0.0000030 = 1.5$$

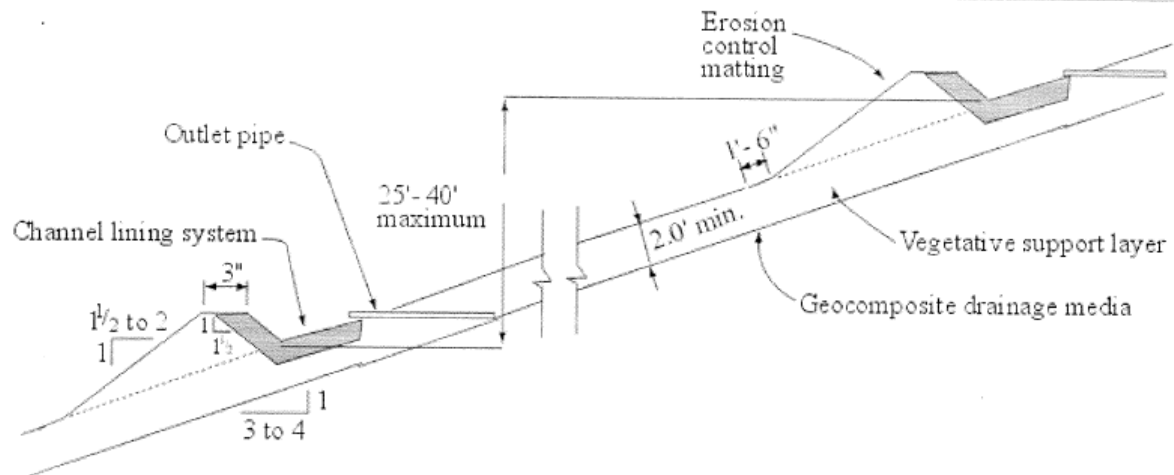
( not including consideration for clogging)

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## Unit Gradient Method - Design Calculator

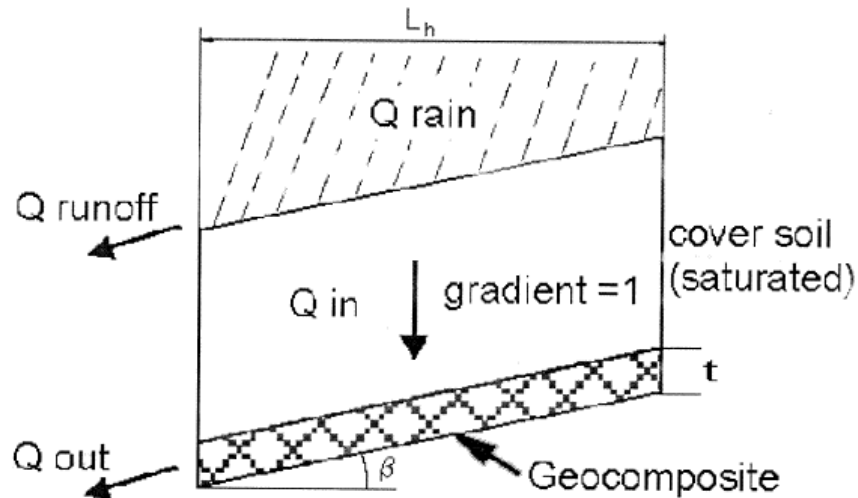
### Problem Statement



The transmissivity of a drainage geocomposite must be great enough to carry all of the infiltrating flow from the soil layer(s) above. If the drainage geocomposite can not carry all the infiltrating water (very long slope, or very permeable cover soil,...); swales can be placed as shown in the above figure. The three conditions for stability are:

1. The interface shear strength of all interfaces is adequate
2. Pore water pressures do not build up and reduce the contact stress between the geomembrane and the soil. The [Seepage Force Stability Calculator](#) can be used to determine the factor of safety of a landfill cover with consideration of seepage forces
3. Landfill gas pressures beneath the liner are vented properly. The [Landfill Gas Pressure Relief Calculator](#) can be used to determine the gas transmissivity of the relief layer. The [Landfill Gas Stability Calculator](#) can be used to verify the factor of safety of a landfill cover subject to landfill gas pressure underneath a geomembrane liner.

This webpage determines the ultimate transmissivity sufficient to transmit all incoming flow within the thickness of the geocomposite; i.e. maximum head < geonet thickness; therefore seepage forces in the cover soil will be zero.



With Darcy's law:

$$Q = k * i * A$$

Inflow of water in the geocomposite

$$Q_{in} = k_{veg} * i * A = k_{veg} * 1 * L_h * 1$$

Outflow of water from the geocomposite at the toe of the slope

$$Q_{out} = k_{comp} * i * A = k_{comp} * i * t * 1 = \theta_{required} * \sin \beta$$

Inflow equals outflow (Factor of Safety = 1)

$$Q_{in} = Q_{out}$$

This results in a required transmissivity of the geocomposite of:

$$\theta_{required} = \frac{k_{veg} * L_h}{\sin \beta}$$

Which results in the ultimate transmissivity after multiplying by the Total Serviceability Factor (TSF)

$$\theta_{ultimate} = \theta_{required} * FS_d * RF_{in} * RF_{cr} * RF_{cc} * RF_{dc}$$

**Required Data**

Symbol	Name	Dimensions
$L_h$	Drainage pipe spacing or length of slope measured horizontally	Length
$k_{veg}$	Permeability of the vegetative supporting soil	Length/Time
$S$	The liner's slope, $S = \tan b$	-
$FS_{slope}$	Minimum factor of safety against sliding, for soil/geocomposite or geocomposite/geomembrane interfaces	-

$FS_d$	Overall factor of safety for drainage
$RF_{in}$	Intrusion Reduction Factor
$RF_{cr}$	Creep Reduction Factor
$RF_{cc}$	Chemical Clogging Reduction Factor
$RF_{bc}$	Biological Clogging Reduction Factor

**Input Values**

Note: If you do not wish to perform calculations for 3 cases, please leave default data as is.

	Case 1		Case 2		Case 3	
$S$	5	%	5	%	5	%
$L_h$	30	m	30	m	30	m
$k_{veg}$	0.00001	cm/sec	0.00001	cm/sec	0.00001	cm/sec
$FS_{slope}$	1.5		1.5		1.5	

**Reduction Factors and Safety Factor**

	Case 1	Case 2	Case 3	Surface Water Drains
$RF_{in}$	1.5	1.5	1.5	[1] 1.0 - 1.2
$RF_{cr}$	1.4	1.4	1.4	[2] Calculate $RF_{CR}$
$RF_{cc}$	1.2	1.2	1.2	[3] 1.0 - 1.2
$RF_{bc}$	1.6	1.6	1.6	[3] 1.2 - 3.5
$FS_d$	2	2	2	[4] 2.0 - 10.0

Calculate Transmissivity



[1] Intrusion reduction factor from 100 hour to design life. Giroud et. al (2000)

[2] Creep reduction factor from 100 hour to design life (for instance, 30 years).  $RF_{CR}$  is determined from 10,000 hour compressive creep test, extrapolated to design life, GRI-GC8 (2001).  $RF_{CR}$  is product and normal load specific.

[3] GRI-GC8

[4] FS value = 2-3. Giroud, et. al (2000)

FS value > 10 for filtration and drainage. Koerner (2001)

[5] Note: The calculated transmissivity is corresponding to the case where the seating time is 100 hours and the boundary conditions due to adjacent materials are simulated in the hydraulic transmissivity test.

## Solution

Symbol	Name	Dimensions
gradient	Gradient	
$\theta_{ultimate}$	Ultimate Transmissivity	Length <sup>2</sup> /Time
$\delta_{req'd}$	Minimum interface friction angle	degrees

	Case 1		Case 2		Case 3	
gradient	0.05		0.05		0.05	
$\theta_{ultimate}$	4.84E-004	m <sup>2</sup> /s	4.84E-004	m <sup>2</sup> /s	4.84E-004	m <sup>2</sup> /s
$\delta_{req'd}$	4.29	degrees	4.29	degrees	4.29	degrees

## Material Selection

Follow the GFR link to view our extensive database of geosynthetic materials reprinted with permission of IFAI



## Additional Assistance

If you would like to have Advanced Geotech Systems provide material specifications that meet your performance criteria, please fill in the following fields and click the submit button. All information is kept strictly confidential.

Name *	<input type="text"/>	Comments	<input type="text"/>
Company	<input type="text"/>		
Email Address *	<input type="text"/>		
Phone	<input type="text"/>		
Project Reference	<input type="text"/>		

\*required fields

Submit Design Results

### Sponsored by

The following companies can service any of your geosynthetic drainage material selection needs.



POLY-FLEX, INC.

### References

"GRI-GC8, Determination of the Allowable Flow Rate of a Drainage Geocomposite". Geosynthetics Research Institute, 2001.

"Beyond a factor-of-safety value, i.e., the probability of failure". GRI Newsletter/Report, Vol. 15, no. 3.

"Designing with Geosynthetics". **R.M. Koerner**, Prentice Hall Publishing Co., Englewood Cliffs, NJ, 1998.

"Hydraulic Design of Geosynthetic and Granular Liquid Collection Layers". **J. P. Giroud, J. G. Zornberg and A. Zhao**, *Geosynthetics International*, Vol. 7, Nos 4-5.

"Lateral Drainage Design update - part 2". **G. N. Richardson**, J.P. Giroud and **A. Zhao**, *Geotechnical Fabrics Report*, March, 2002

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GSE STANDARD PRODUCTS

## Product Data Sheet

### GSE FabriNet HF

250 mil GSE FabriNet HF geocomposites typically consist of a 250 mil GSE HyperNet core with a nonwoven geotextile fabric heat-bonded to one or both sides. The geotextile serves as a filter to protect the geonet core from clogging while the geonet provides a path for the fluids (liquids and gases). The 250 mil GSE HyperNet core is manufactured in the same manner as standard GSE HyperNet but is designed to handle higher flow requirements, as well normal loads, such as those expected in landfill expansions.

#### Product Specifications

TESTED PROPERTY	TEST METHOD	FREQUENCY	MINIMUM AVERAGE VALUE <sup>(d)</sup>		
Geocomposite			6 oz/yd <sup>2</sup>	8 oz/yd <sup>2</sup>	10 oz/yd <sup>2</sup>
Product Code:			F52060060S	F52080080S	F52100100S
Transmissivity <sup>(a)</sup> , m <sup>2</sup> /sec	ASTM D 4716-00	1/540,000 ft <sup>2</sup>	5x10 <sup>-4</sup>	5x10 <sup>-4</sup>	5x10 <sup>-4</sup>
Ply Adhesion, lb/in average	GRI GC-7	1/50,000 ft <sup>2</sup>	1.0	1.0	1.0
Roll Width, ft (m)			14.5 (4.4)	14.5 (4.4)	14.5 (4.4)
Roll Length, ft (m)			230 (70.1)	190 (57.9)	180 (54.9)
Roll Area, ft <sup>2</sup> (m <sup>2</sup> )			3,335 (310)	2,755 (256)	2,610 (242)
Geonet core <sup>(b)</sup>					
Transmissivity <sup>(a)</sup> , m <sup>2</sup> /sec	ASTM D 4716-00		3x10 <sup>-3</sup>	3x10 <sup>-3</sup>	3x10 <sup>-3</sup>
Thickness, mil (mm)	ASTM D 5199	1/50,000 ft <sup>2</sup>	250 (6.3)	250 (6.3)	250 (6.3)
Density, g/cm <sup>3</sup>	ASTM D 1505	1/50,000 ft <sup>2</sup>	0.94	0.94	0.94
Tensile Strength (MD), lb/in (N/mm)	ASTM D 5035	1/50,000 ft <sup>2</sup>	55 (9.6)	55 (9.6)	55 (9.6)
Carbon Black Content, %	ASTM D 1603	1/50,000 ft <sup>2</sup>	2.0	2.0	2.0
Geotextile (prior to lamination) <sup>(b,c)</sup>					
Mass per Unit Area, oz/yd <sup>2</sup> (g/m <sup>2</sup> )	ASTM D 5261	1/90,000 ft <sup>2</sup>	6 (200)	8 (270)	10 (335)
Grab Tensile, lb (N)	ASTM D 4632	1/90,000 ft <sup>2</sup>	170 (755)	220 (975)	260 (1,155)
Puncture Strength, lb (N)	ASTM D 4833	1/90,000 ft <sup>2</sup>	90 (395)	120 (525)	165 (725)
AOS, US Sieve (mm)	ASTM D 4751	1/540,000 ft <sup>2</sup>	70 (0.212)	80 (0.180)	100 (0.150)
Flow Rate, gpm/ft (l/min/m <sup>2</sup> )	ASTM D 4491	1/540,000 ft <sup>2</sup>	110 (4,480)	110 (4,480)	85 (3,460)
UV Resistance, % Retained	ASTM D 4355 (after 500 hours)	once per formulation	70	70	70

**NOTES:**

- <sup>(a)</sup> Gradient of 0.1, normal load of 10,000 psf, water at 70° F between stainless steel plates for 15 minutes.
- <sup>(b)</sup> Component properties prior to lamination.
- <sup>(c)</sup> Several geotextiles are available and may be supplied as determined by GSE.
- <sup>(d)</sup> These are MARV values and are based on the cumulative results of specimens tested and as determined by GSE.

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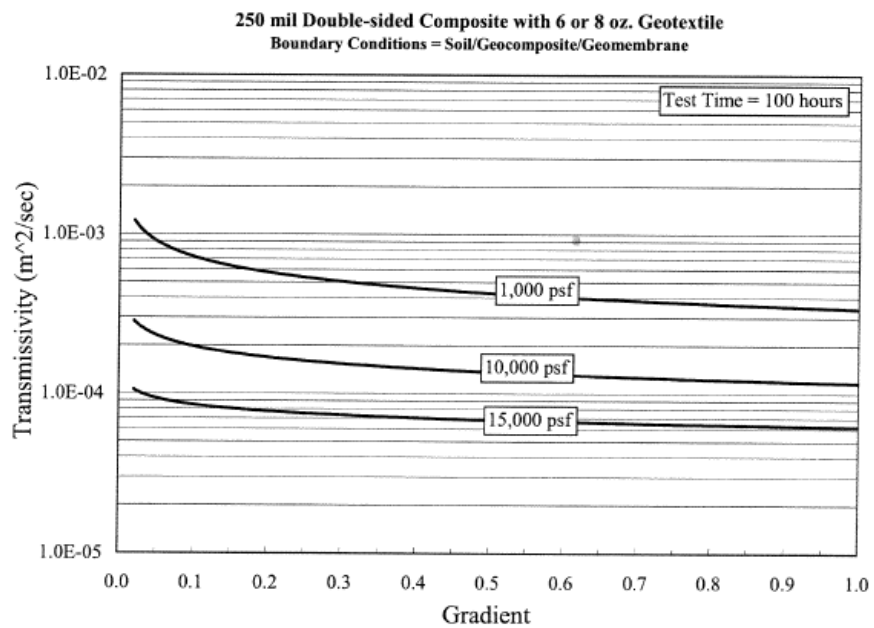


Figure A-6 100-hour transmissivity of a 250 mil biplanar geonet under soil/geocomposite/geomembrane boundary conditions.